

69-207

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2 An Iterative Digital Model for Aquifer Evaluation
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5 By
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11 PURPOSE
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14 This program simulates the response of a confined or unconfined
15 aquifer to pumping at a constant rate from one or more wells. The
16 ground-water reservoir may be irregular in shape and non-homogeneous
17 with infiltration from one or more lakes and streams. The program is
18 written in FORTRAN IV for the IBM 360 system. (See Appendix for
19 program listing).
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2 THEORETICAL DEVELOPMENT
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5 The differential equation for nonsteady flow of a
6 compressible fluid in an elastic non-homogeneous porous medium
7 can be written (Pinder and Bredehoeft, 1968)

8 $\frac{\partial}{\partial x_i} (T_{ij} \frac{\partial h}{\partial x_j}) = S \frac{\partial h}{\partial t} + w(x, y, t) \quad (1)$

9
10 where T_{ij} is the transmissivity tensor (L^2/T);
11 h is the hydraulic head (L);
12 S is the storage coefficient (dimensionless);
13 t is time (T);
14 w is the volume flux per unit area (L/T).
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If the coordinate axes are aligned with the principal directions of the transmissivity tensor the finite-difference approximation to (1) may be written (see Fig.1)

$$T'_{xx}(i-\frac{1}{2}, j) \left(\frac{h_{i-1,j,k} - h_{i,j,k}}{\Delta x_i} \right)$$

$$+ T'_{xx}(i+\frac{1}{2}, j) \left(\frac{h_{i+1,j,k} - h_{i,j,k}}{\Delta x_i} \right)$$

$$+ T'_{yy}(i, j-\frac{1}{2}) \left(\frac{h_{i,j-1,k} - h_{i,j,k}}{\Delta y_j} \right)$$

$$+ T'_{yy}(i, j+\frac{1}{2}) \left(\frac{h_{i,j+1,k} - h_{i,j,k}}{\Delta y_j} \right)$$

$$= S \left(\frac{h_{i,j,k} - h_{i,j,k-1}}{\Delta t} \right)$$

$$+ \frac{q_w(i,j)}{\Delta x_i \Delta y_j} - K_s \left(\frac{2H_r(i,j) - h_{i,j,k} - h_{i,j,k-1}}{2m_r(i,j)} \right)$$

where

$$T'_{xx}(i+\frac{1}{2}, j) = \frac{2T_{xx}(i,j)T_{xx}(i+1,j)}{T_{xx}(i,j)\Delta x_{i+1} + T_{xx}(i+1,j)\Delta x_i} = \text{the harmonic mean of}$$

mean of

$$\frac{T_{xx}(i,j)}{\Delta x_i}, \quad \frac{T_{xx}(i+1,j)}{\Delta x_{i+1}}$$

1 i is the index in the x dimension;
2 j is the index in the y dimension;
3 k is the index in time;
4 K_s is the hydraulic conductivity of the stream bottom (L/T);
5 m_r is the thickness of the stream bottom (L);
6 $q_w(i,j)$ is the rate of withdrawal or injection at the node $x_i y_j (L^3/T)$;
7 Δt is the time increment (T);
8 $\Delta x, \Delta y$ are space increments (L);
9 H_r is the hydraulic head in the lakes and streams.
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1 In an unconfined aquifer the transmissivity is a function of
2 time as well as of space. In order to approximate equation (2)
3 without generating a non-linear set of finite-difference equations,
4 the head from the preceding time step is used, and the transmissivity
5 is given by

6

7 $T_{i,j,k} = K_{i,j} h'_{i,j,k-1}$

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11 where K is the hydraulic conductivity (L/T);

12 h' is the saturated thickness of the aquifer (L).

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3 METHOD OF ANALYSIS
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6 A rectangular net or grid as indicated in figure 1 is
7 superposed on a plan view of the groundwater reservoir.
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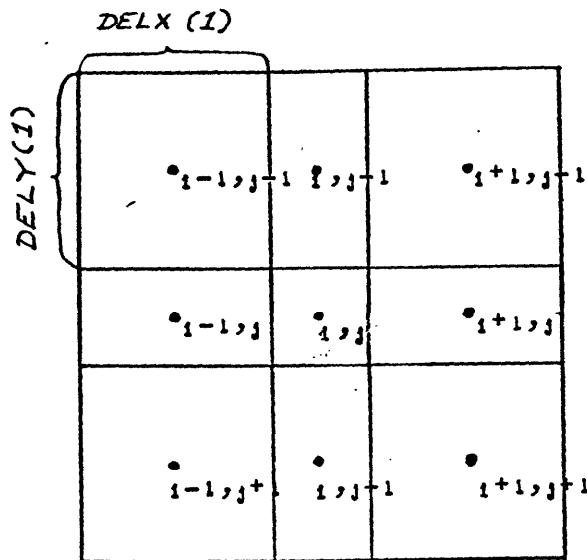


Fig. 1. Nodal array for digital model.

1 At each node the transmissivity, storage coefficient, and initial
2 head are recorded. The pumping rate is recorded at each node where a
3 well is located and the hydraulic conductivities of stream or lake
4 beds where they occur. In the case of an unconfined aquifer
5 the transmissivity is replaced by the hydraulic conductivity and the
6 elevation of the base of the aquifer is also recorded. A parameter
7 card is prepared providing the dimensions of the grid, the head in
8 the stream, the thickness of the stream or lake beds, information
9 concerning the maximum duration of pumping and other constants used
10 in the computational scheme. All computer input and output is in a
11 consistent set of units; the program is set-up to use feet and
12 seconds.

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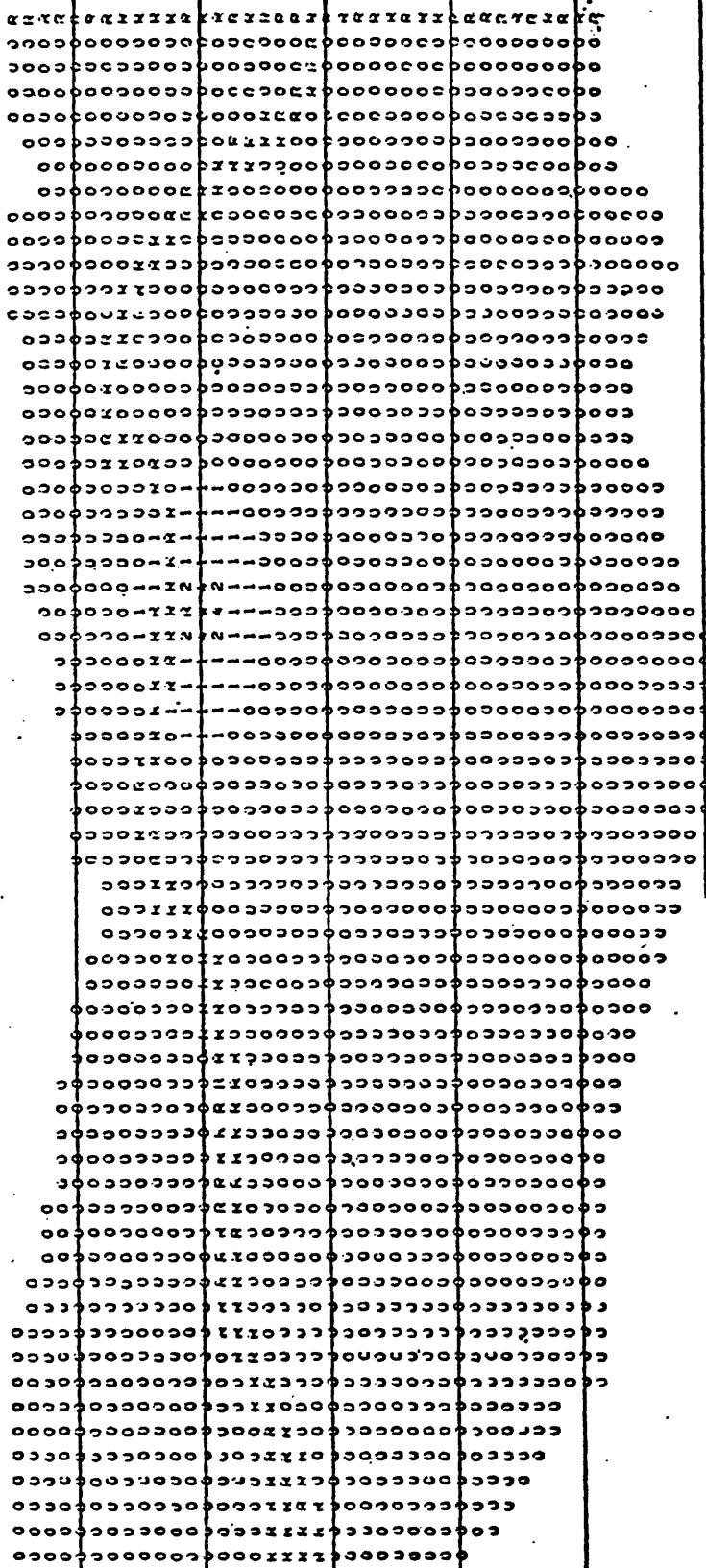
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2 The objective of the program is to solve equation (2) implicitly
3 for the desired boundary conditions at each node on the grid after
4 Δt seconds of pumping. These values are then used as initial
5 conditions for the calculation of the head values at time $2\Delta t$;
6 this procedure is continued until the reservoir has been pumped the
7 desired length of time. The size of the time step is increased
8 exponentially. Numerical values and an alphameric contour map of
9 the drawdown in the aquifer are printed at selected time steps
10 (see Fig. 2).

11 _____
12 Figure 2 -- Alphameric contour map of drawdown at Antigonish
13 Nova Scotia
14 _____

15 In order to determine the precision of the calculations, a mass
16 balance may also be computed at this time if there is no infiltration
17 into the system.
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ITERATION NUMBER 15
SIZE OF TIME STEP IN SECONDS = 12.33
DURATION OF PUMPING AT THIS PINTOUT IN SECONDS = 17.5
MINUTES = 0.291
HOURS = 0.00555-02
DAYS = 0.0202L-03

ALPHADÉTIC CUNTOURS FOR DRAWDOWN



LEGEND...
DRAWDOWN FROM 0-35 FEET REPRESENTED BY SYMBOLS 0-2
DRAWDOWN GREATER THAN 35 FEET REPRESENTED BY THE SYMBOL *
CONE OF IMPRESSION INDICATED BY SYMBOL A
LETTER R INDICATES AREA OF RECHARGE
LETTER W INDICATES LOCATION OF A WELL

1 The iterative alternating-direction implicit procedure
 2 is the mathematical technique used to solve the N
 3 simultaneous equations where N is the number of nodes in
 4 the matrix. Hydraulic head values are calculated for each
 5 node in the matrix by solving alternately the equations for
 6 rows and columns implicitly. Equation (2) for the
 7 calculation of rows is

$$\begin{aligned}
 & T'_{xx}(i-\frac{1}{2}, j) \left(\frac{h_{i-1,j,k}^n - h_{i,j,k}^n}{\Delta x_i} \right) \\
 & + T'_{x\dot{x}}(i+\frac{1}{2}, j) \left(\frac{h_{i+1,j,k}^n - h_{i,j,k}^n}{\Delta x_i} \right) \\
 & + T'_{y\dot{y}}(i, j-\frac{1}{2}) \left(\frac{h_{i,j-1,k}^{n-1} - h_{i,j,k}^{n-1}}{\Delta y_j} \right) \\
 & + T'_{yy}(i, j+\frac{1}{2}) \left(\frac{h_{i,j+1,k}^{n-1} - h_{i,j,k}^{n-1}}{\Delta y_j} \right) \\
 & = S \left(\frac{h_{i,j,k}^n - h_{i,j,k-1}}{\Delta t} \right) + \frac{q_w(i,j)}{\Delta x_i \Delta y_j} \\
 & - K_s \left(\frac{2H_r(i,j) - h_{i,j,k}^n - h_{i,j,k-1}}{2m_r(i,j)} \right) \\
 & + I(h_{i,j,k}^n - h_{i,j,k}^{n-1})
 \end{aligned}$$

22 where I is a normalized iteration parameter;
 23 n is the index indicating iteration number.
 24

25-

1 At each time step the row and column equations are solved
2 alternately until the greatest head difference between a row and
3 column computation at any node is less than a prescribed error
4 criteria. When this closure is achieved the program begins solving
5 for the head at the new time $t + \Delta t$. The procedure is
6 continued until the desired period of analysis has been simulated or
7 the aquifer becomes dewatered. When any condition arises which
8 terminates computation the head matrix and the elapsed simulation
9 period are punched on cards. These cards can be used as input if
10 it is necessary to extend the period of analysis at a later time.

11 For a detailed explanation of the technique used for solving
12 the finite-difference equations see Douglas and Rachford (1956).

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1 APPLICATION

2 The following section describes the preparation of parameter
3 and data cards. The arrangement of the assembled program deck is
4 shown in figure 3.

5 -
6 Figure 3. Assembled program deck.

7 -
8 Parameter cards

9 All values are right justified in the data field as indicated
10 - in Fig. 4.

11 -
12 Figure 4. Sample coding form.

13 -
14 Card 1

<u>Column</u>	<u>Variable</u>	<u>Contents</u>
1 - 10	TMAX	simulation period in hours
11 - 20	DIML	number of nodes in a column of the matrix *
21 - 30	DIMW	number of nodes in a row of the matrix *
31 - 40	NUMT	maximum number of time steps *
41 - 50	QRE	vertical leakage into the aquifer in feet per second
51 - 60	DELT	initial time increment in seconds

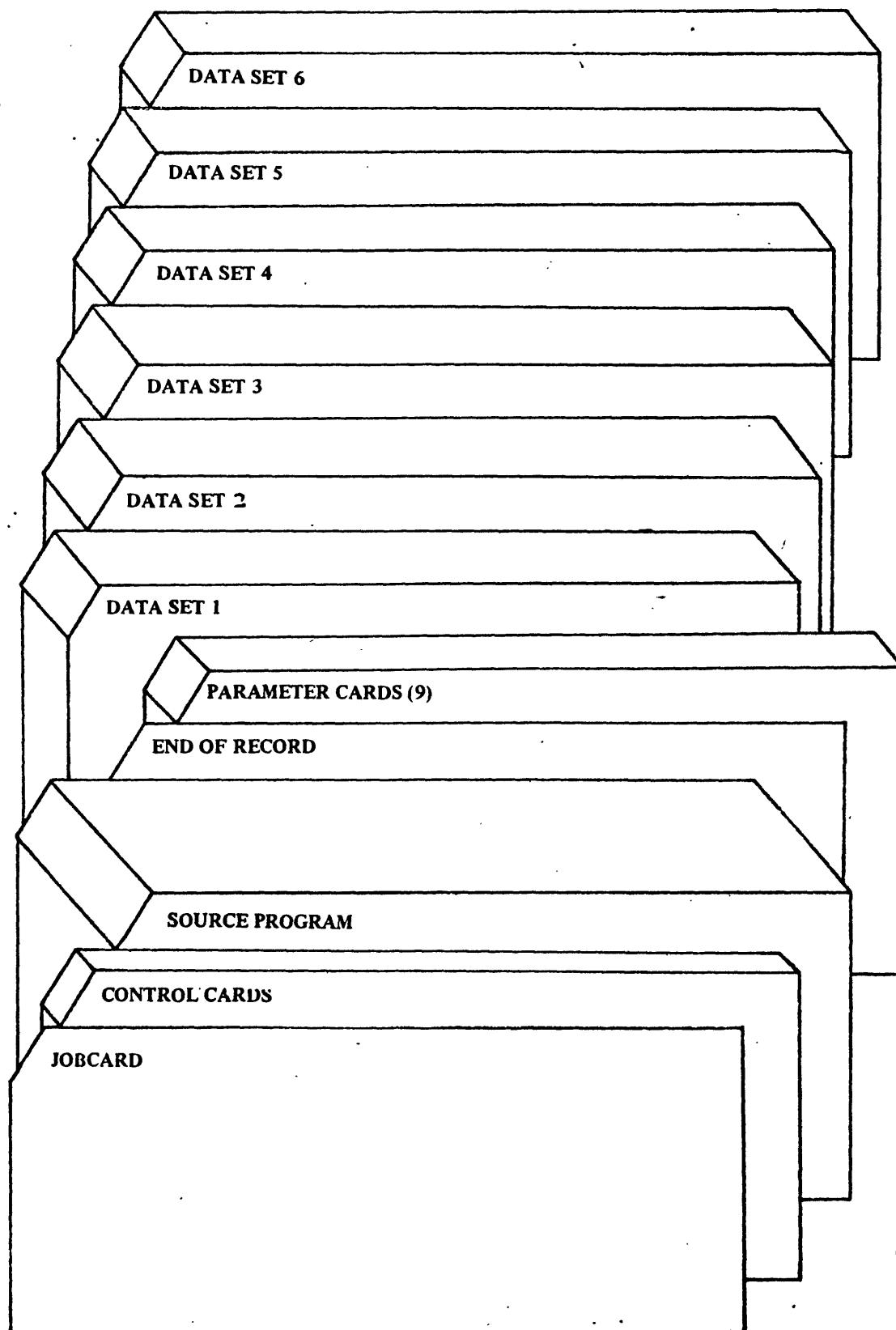


Fig. 3

PROGRAMMER	DIVISION
LOCATION	PHONE
IDENTIFICATION.	INFORMATION.
INTERACTIVE DIGITAL MODEL FOR AQUIFER EVALUATION SAMPLE CODING FORM	
COMPUTER CODING FORM	
U. S. DEPARTMENT OF THE INTERIOR GEOLOGICAL SURVEY PROJECT	
SHEET 1 OF 2	

TMAX	DIML	DI/MV	NUMT	QRE	DELT	FACP
M						
10000.	KTH	FACTOR	LENGTH	ERR	0.	FACS
M						
10.	RIVER	FACT	SPACING		.001	.001
1.	1.	1.	1.	1.	1.	1.
SUM 100.						
PUNCH 0.						
WATERTABLE						
CONTOUR						
NUMERIC						
CHECK						
DATA SET 1; DELX,DELY- 100. 50.						
DATA SET 2; STRT- 100. 100.						
1	2	3	4	5	6	7
8	9	10	11	12	13	14
15	16	17	18	19	20	21
22	23	24	25	26	27	28
29	30	31	32	33	34	35
36	37	38	39	40	41	42
43	44	45	46	47	48	49
50	51	52	53	54	55	56
57	58	59	60	61	62	63
64	65	66	67	68	69	70
71	72	73	74	75	76	77
78	79	80	81	82	83	84
85	86	87	88	89	90	91
92	93	94	95	96	97	98
99	100	101	102	103	104	105

0 = ZERO 1 = ONE 2 = TWO 3 = ALPHA O 4 = ALPHA I 5 = ALPHA Z 6 = SLASH 7 = VERT. BAR 8 = MINUS 9 = TWO =HORZ. BAR =VERT. BAR =MINUS

DATA CODING FORM
 JULY 1977

Fig. 4

DIVISION		U S DEPARTMENT OF THE INTERIOR GEOLOGICAL SURVEY		PROGRAM NO.																																																			
PHONE		COMPUTER CODING FORM		PROJECT																																																			
INVESTIGATION		INTERACTIVE DIGITAL MODEL FOR AQUIFER EVALUATION		STATEMENT IDENTIFICATION																																																			
SAMPLE CODING FORM																																																							
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13		14		15																																																			
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67		68		69																																																			
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73		74		75																																																			
76		77		78																																																			
79		80																																																					
DATA SET 3; RATE 0. -1. 0. 0. 0. 0.		DATA SET 4; PERM 0. L. 0. 0. 0. 0.		DATA SET 5; BOTTOM 0. 0. 0. 0. 0.		DATA SET 6; S 1. 1. 1. 1. 1.		DATA SET 7; T 1. 1. 1. 1. 1.		DATA SET 8; K 1. 1. 1. 1. 1.		DATA SET 9; R 1. 1. 1. 1. 1.		DATA SET 10; C 1. 1. 1. 1. 1.		DATA SET 11; H 1. 1. 1. 1. 1.		DATA SET 12; D 1. 1. 1. 1. 1.		DATA SET 13; I 1. 1. 1. 1. 1.		DATA SET 14; J 1. 1. 1. 1. 1.		DATA SET 15; L 1. 1. 1. 1. 1.		DATA SET 16; M 1. 1. 1. 1. 1.		DATA SET 17; N 1. 1. 1. 1. 1.		DATA SET 18; O 1. 1. 1. 1. 1.		DATA SET 19; P 1. 1. 1. 1. 1.		DATA SET 20; Q 1. 1. 1. 1. 1.		DATA SET 21; R 1. 1. 1. 1. 1.		DATA SET 22; S 1. 1. 1. 1. 1.		DATA SET 23; T 1. 1. 1. 1. 1.		DATA SET 24; U 1. 1. 1. 1. 1.		DATA SET 25; V 1. 1. 1. 1. 1.		DATA SET 26; W 1. 1. 1. 1. 1.		DATA SET 27; X 1. 1. 1. 1. 1.		DATA SET 28; Y 1. 1. 1. 1. 1.		DATA SET 29; Z 1. 1. 1. 1. 1.		DATA SET 30; B 1. 1. 1. 1. 1.	

0 = ZERO 1 = ALPHA 0 2 = ALPHA 1 3 = ONE 4 = TWO 5 = ALPHA Z 6 = SLASH 7 = VERT. BAR 8 = MINUS 9 = HORZ. BAR

█ = PROGRAM CODING FORM

GPO : 1960 O - 266 - 104

Fig. 4

1 Card 2

2 1 - 10 M thickness of stream or lake beds in feet
3 11 - 20 KTH number of time steps between printouts *
4 21 - 30 FACTOR multiplier for values of hydraulic
5 conductivity of stream or lake beds
6 31 - 40 LENGTH number of iteration parameters *
7 41 - 50 ERR error criteria for closure
8 51 - 60 FACS multiplier for storage coefficient
9 61 - 70 FACB multiplier for aquifer base bottom
10 elevation
11 71 - 80 FACP multiplier for hydraulic conductivity
12 of aquifer

13 Card 3

14 1 - 10 FACT multiplier for transmissivity
15 11 - 20 RIVER hydraulic head in river in feet
16 21 - 30 SPACNG contour interval in feet

17 Card 4

18 1 - 10 SUM elapsed time at beginning of computations
19 (usually 0. unless provided as
20 punched output)

21 Card 5

22 1 - 5 PUNCH indicator for punched output; if punched
23 output is desired at termination of
24 computations write PUNCH, otherwise
25 leave card blank.

1 Card 6

2 1 - 10 WATER indicator of water-table conditions, if
3 water-table conditions are
4 encountered write WATERTABLE:
5 otherwise leave card blank.

5- Card 7

6 1 - 7 CONTR indicator of contoured printout;
7 if an alphanumeric contour map of
8 hydraulic head is desired write
9 CONTOUR; otherwise leave this
10 card blank.

9 Card 8

10 1 - 7 NUM indicator of numerical printout; if the
11 numerical head values are desired
12 write NUMERIC, otherwise leave this
13 card blank.

13 Card 9

14 1 - 5 CHCK indicator of computational check; if a
15 mass balance on the system is
16 desired write CHECK; otherwise
17 leave card blank.

18 * values are fixed point; they must be right justified and not
19 include a decimal point.

1 Data Cards

2 Set 1

3 Distance between nodes in the prototype in feet: the distance
4 between adjacent nodes is recorded as indicated in figure 1. The
5 values in the x direction, $DELX$, are recorded first and are followed,
6 beginning on a new card, by the values in the y direction, $DELY$.
7 As indicated in figure 1 the values for $DELY$ and $DELX$ must remain
8 constant for each row and column respectively. Each value may
9 occupy 10 spaces including the decimal point.

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1 Set 2

2 Initial hydraulic head, STRT: the initial hydraulic head values are
3 recorded from left to right along the rows beginning with the top row.
4 Each row begins on a new card and each value may occupy 10 spaces
5 including a decimal point (there will be a maximum of eight values
6 recorded on each card). The hydraulic head values punched at the
7 termination of the program can be read as initial hydraulic head
8 values.

9 Set 3

10 Locations of wells, lakes, and streams, RATE: the locations of wells,
11 lakes, and streams are recorded left to right along the rows,
12 beginning with the top row. Each row begins on a new card and each
13 value may occupy only 4 spaces including a negative sign and decimal
14 point (no sign is necessary for a positive number). At any node
15 where a lake or stream occurs the hydraulic conductivity of the bed
16 divided by the hydraulic conductivity multiplier is recorded. At
17 any node where a well is located the pumping rate in cfs is recorded
18 preceded by a negative sign.

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1 Set 4

2 Hydraulic conductivity: PERM or transmissivity T: this data set
3 consists of the hydraulic conductivity values in the case of a water
4 table problem or transmissivity values in a confined aquifer situation.
5 The values divided by the appropriate multiplier are recorded left to
6 right along the rows beginning with the top row. Each row begins on
7 a new card and each value may occupy up to 4 spaces including the
8 decimal point.

9 Set 5

10 Aquifer base elevation, BOTTOM: this data set consists of the
11 elevation of the base of the aquifer measured from the same reference
12 datum as the initial head, divided by an appropriate multiplier.
13 This data set is omitted if a confined-aquifer problem is considered.
14 The values are recorded left to right along the rows beginning with
15 the top row. Each row begins on a new card and each value may occupy
16 up to 4 spaces including the decimal point.

17 Set 6

18 Storage coefficient values, S: this data set consists of the storage
19 coefficient at each node, divided by the appropriate multiplier.
20 The values are recorded left to right along the rows, beginning with
21 the top row. Each row begins on a new card and each value may occupy
22 up to 4 spaces including the decimal point.

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2 PROGRAM FEATURES

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4 Number of nodes:

5 The dimensions of the nodal array considered in an aquifer problem
6 generally depend upon the storage capacity in the computer and
7 available funds. The larger the nodal array the more computer time
8 is required to simulate a specified period of analysis. This
9 program permits arrays up to 50 X 50 nodes.

10 Drawdown near the pumping well:

11 Withdrawal from a well is assumed to occur over the area of
12 influence of the well node. Drawdown values within one node of
13 the well, therefore, will not accurately represent field responses.
14 The use of smaller space increments near the well will improve
15 the accuracy of the solution.

16 Advantages of iterative technique:

17 Certain limitations inherent in the alternating direction
18 implicit technique are overcome by introducing a relaxation
19 factor and iteration. Some of the important advantages of
20 an iterative analysis are:

- 1 i. abrupt changes in transmissivity between nodes is
- 2 permitted,
- 3 ii. variable space increments may be used,
- 4 iii. any time increment Δt may be used,
- 5 iv. the elliptic form of the flow equation can be treated
- 6 directly (the steady-state problem),
- 7 v. the iterative procedure requires less computer time
- 8 when periods extending tens or hundreds of years are
- 9 simulated.

10- Multiplication Constants

11 Multiplication constants (FACTOR, FACB, FACS, FACP, FACT) are used
12 to decrease the number of data cards when very small or very large
13 numbers must be read. For example, if it was necessary to read a
14 storage coefficient value of .0001 the multiplication factor FACS
15 could be set equal to .0001 and 1. would be recorded on the data
16 card; the program would convert the 1. to .0001 during execution.

17 Program Diagnostics

18 Two program diagnostics may be generated if an abnormal program
19 termination occurs. The message EXCEEDED PERMITTED NUMBER OF
20 ITERATIONS is printed when convergence is not achieved within
21 100 iterations. This situation generally arises when an error in
22 data input results in an impossible physical problem. The message
23 WELL GOES DRY occurs when the cone of depression in an unconfined
24 aquifer drops below the impermeable aquifer base.

1 Iteration Parameters

2 The choice of the number of iteration parameters, LENGTH, and the
3 convergence criteria, ERR, depends upon the physical problem
4 considered. Three to seven iteration parameters and a value for
5 ERR of .001 should provide satisfactory results in most problems.

6 Mass Balance

7 The CHECK option permits the calculation of a mass balance when
8 there is no infiltration into the system. The volume of water
9 removed from the aquifer is computed by integration over the cone of
10 depression and by calculating the volume removed from the wells.
11 These values are compared and the deviation expressed as a percent of
12 the volume pumped. If infiltration does occur the deviation is a
13 measure of the volume of water entering the system.

14 This program is not designed to simulate all of the complicated
15 hydrologic problems encountered in the field. It is intended as a
16 starting point from which more complex models can be developed.

1
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3
4

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6 solution of head conduction problems in two or three space
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8
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10 Digital Computer for Aquifer Evaluation; Water Resources
11 Research, Vol. 4, No. 5, pp. 1069-1093.
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C   TO PROVIDE THE POTENTIAL DISTRIBUTION IN A CONFINED AQUIFER AFTER A 1
C   A DESIGNATED PERIOD OF TIME. A 2
***** A 3
***** A 4
***** A 5
***** A 6
***** A 7
***** A 8
***** A 9
***** A 10
***** A 11
***** A 12
***** A 13
***** A 14
***** A 15
***** A 16
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***** A 50

C   METHODS A
C   THE ALTERNATING DIRECTION IMPLICIT TECHNIQUE IS USED TO SOLVE A
THE LINEAR EQUATIONS. A

C   DESCRIPTION OF PARAMETERS A
A=A=COEFFICIENT IN FINITE DIFFERENCE EQUATION A
B=B=COEFFICIENT IN FINITE DIFFERENCE EQUATION A
RE(I,J)=DUMMY VARIABLE DEFINED AS C/W A
RUT(I,J)=ELEVATION OF BOTTOM A
C=C=COEFFICIENT IN FINITE DIFFERENCE EQUATION WHOSE VALUES A
D=DUMMY VARIABLE REPRESENTING PARAMETERS OF EQUATION WHOSE VALUES A
ARE KNOWN FROM PREVIOUS ITERATION A
DAYS=NUMBER OF DAYS SINCE PUMPING STARTED A
DELT=LENGTH OF INITIAL TIME STEP (SECONDS) A
DELX=DISTANCE BETWEEN NODES IN THE PROTOTYPE IN THE X DIRECTION A
(Feet) A
DELY=DISTANCE BETWEEN NODES IN THE PROTOTYPE IN THE Y DIRECTION A
(FEET) A
DIML=NUMBER OF NODES IN COLUMN OF MATRIX A
DIMX=NUMBER OF NODES IN ROW OF MATRIX A
ER3=CLOSURE CRITERIA FOR ITERATION A
FACT=MULTIPLICATION FACTOR FOR ADJUSTING STORAGE COEFFICIENT A
FACTD=MULTIPLICATION FACTOR FOR BOTTOM ELEVATION A
FACTP=MULTIPLICATION FACTOR FOR HYDRAULIC CONDUCTIVITY A
FACTS=MULTIPLICATION FACTOR FOR STORAGE COEFFICIENT A
FACTT=MULTIPLICATION FACTOR FOR TRANSMISSIVITY A
G=DUMMY VARIABLE DEFINED AS (D-A*(J-1))/W A
HRS=NUMBER OF HOURS SINCE PUMPING STARTED A
IRK=ITERATION PARAMETER A
KEE?(I,J)=STORAGE MATRIX FOR PHI VALUES A
KTH=NUMBER OF TIME STEPS BETWEEN PRINTOUTS A
N=THICKNESS OF STREAM BED OR LAKE BOTTOM (FEET) A
NMT=MAXIMUM NUMBER OF TIME STEPS A
PHI(I,J)=HEAD IN AQUIFER AT NODE (I,J) IN FEET A
PPNT(I,J)=ARRY FOR PRINTING OUT HEAD MATRIX IN CONTOUR FORM A
QIN=RECHARGE FLUX FROM THE CONFINING LAYER (CUBIC FEET PER SECOND) A
RATE(I,J)=RATE OF PUMPING IN CFS IF NEGATIVE A
HYDRAULIC CONDUCTIVITY OF STREAM OR LAKE BOTTOM IF A
POSITIVE A
RH0=DUMMY VARIABLE DEFINED AS S(I,J)/DELT A
RIVER=HEAD IN RIVER A
RE=VARIABLE ACCOUNTING FOR VERTICAL LEAKAGE A
RN=VARIABLE ACCOUNTING FOR PUMPING A
SI(I,J)=STORAGE COEFFICIENT (DIMENSIONLESS) A
STRY(I,J)=INITIAL VALUE OF HYDRAULIC HEAD IN AQUIFER A
SUM=DURATION OF PUMPING IN SECONDS A

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C SYM(J)=LINEAR ARRAY CONTAINING SYMBOLS FOR PRINTING OUT HEAD

C NATP(X
C T(I,J)=TRANSMISSIVITY (FEET ** 2/SECOND)
C TEMP(I)=TEMPORARY LOCATION OF CURRENTLY CALCULATED VALUES OF HEAD
C TMAY=MAXIMUM ALLOTTED PERIOD OF PUMPING
C W=DUSK/W VAPORABLE DEFINED AS D-A*B(E(J-1))
C
C
C COMMON PHI,SUM,DELX,DELY,DIMW,RATE,S,STRT,SPACING
C DIMENSION S(50,50), PERM(50,50), BOTTOM(50,50), RATE(50,50), KEEP(60
C 150,50), C(50), TEMP(50), BE(50), RHOP(125), CHK(10), STRT(50,50), T
C 2(50,50), PHI(50,50)
C DIMENSION DELX(50), DELY(50)
C
C REAL MINS,W,K
C
C REAL A,KEEP,IMK,NUM
C
C INTEGER DINL,DIMW
C
C DOUBLE PRECISION PHI,KEEP,D,G,TEMP,RE,W,T1,T2,T3,T4,RHO,A,B,C,DELT
C
C PHOP,PARAM,IMK,RIVER,CHK,PNCN,WATER,CONTR,NUM,CHK
C
C
C DATA CHK(1)/5HPUUNCH/,CHK(2)/8HKAERTAB/,CHK(3)/7HCONTOUR/,CHK(4)/7
C 1HNUMERIC/,CHK(5)/5HCHECK/
C
C
C READ (5,520) TMAX,DIML,DIMW,NUMT,QRE,DELT,M,KTH,FACTOR,LENGTH,ERR,
C 1FACTS,FACP,FACT,RIVER,SPACING
C
C READ (5,510) SUM
C
C READ (5,600) PNCN,WATER,CONTR,NUM,CHK
C
C PFAO (5,630) (NEXI(J),J=1,DIMW)
C PFAO (5,640) (NFLY(I),I=1,DIML)
C
C DO 10 J=1,DIML
C
C READ (5,620) (STRT(I,J),J=1,DIMW)
C
C 10 J=1,DIMW
C
C IF(I,J)=STRT(I,J)
C
C 20 IF (PAT(I,J).GT.C) RATE(I,J)=RATE(I,J)*FACTOR
C
C IF (WATER.NE.CHK(2)) GO TO 50
C
C DO 30 I=1,DIML
C
C PFAO (5,540) (PERM(I,J),J=1,DIMW)
C
C 30 IF(I,J)=PFAO(I,J)*FACP
C
C PFAO (5,530) (BOTTOM(I,J),J=1,DIMW)
C
C 40 ACTION(I,J)=BOTTOM(I,J)*FACB
C
C 50 TO 70
C
C 60 DO 60 I=1,DIML
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C 70
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C 80
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C 90
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C 100

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      READ (5,530) (T(I,J),J=1,DIMW)
      DO 60 J=1,DIMW
      60 T(I,J)=T(I,J)*FACT
      DO 70 I=1,DIML
      70 S(I,J)=S(I,J),J=1,DIMW
      READ (5,530) (S(I,J),J=1,DIMW)
      DO 80 J=1,DIMW
      80 S(I,J)=S(I,J)*FACT
      C
      C
      C PRINT PARAMETER VALUES
      WRITE (6,550) DELT,TMAX,NUMT,QRE,DIMW,DIML,ERR,LENGTH,ERR,FACTOR,FACS
      1FACB,FACT,FACT,RIVER,M,KTH
      WRITE (6,690) (DELY(J),J=1,DIMW)
      WRITE (6,700) (DELY(I),I=1,DIML)
      THMAX=TMAX*3600.
      IF (WATER<0.CHK(2)) GO TO 100
      WRITE (6,560)
      20 DO I=1,DIML
      20 90 WRITE (6,57) 1,(T(I,J),J=1,DIMW)
      90 WRITE (6,57C) 1,(T(I,J),J=1,DIMW)
      100 WRITE (6,590)
      DO 110 I=1,DIML
      110 WRITE (6,57C) 1,(S(I,J),J=1,DIMW)
      WRITE (6,540)
      DO 120 I=1,DIML
      120 WRITE (6,57C) 1,(RATE(I,J),J=1,DIMW)
      IF (WATER>0.CHK(2)) GO TO 150
      WRITE (6,630)
      DO 130 I=1,DIML
      130 WRITE (6,57C) 1,(PERM(I,J),J=1,DIMW)
      WRITE (6,640)
      DO 140 I=1,DIML
      140 WRITE (6,570) 1,(BOTTOM(I,J),J=1,DIMW)
      150 JNO1=DIMW-1
      C
      C
      C COMPUTE ITERATION PARAMETERS
      COMPUTE HMIN
      HMIN=2.
      XVAL=3.1415**2/(2.*DIMW**2)
      YVAL=3.1415**2/(2.*DIML**2)
      DO 160 I=2,DIML
      160 J=2,DIMW
      IF (T(I,J).EQ.0.) GO TO 160
      XPART=YVAL*(1/(1+DELX(J)**2/DELY(I)**2))
      YPART=YVAL*(1/(1+DELY(I)**2/DELX(J)**2))
      HMIN=AMIN(HMIN,XPART,YPART)
      160 CONTINUE
      ALPHA=EXP(ALOG(1/HMIN)/(LENGTH-1))

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A 151
DO 170 NTIME=2,LENGTH
170 RHOP(NTIME)=RHOP(INTIME-1)*ALPHA
PARAM=RHOP(1)
      WRITE (6,650) (RHOP(J),J=1,LNGTH)
      KT=0
      TEST=0
      C
      C
      IF TEST EQUALS 1 CONTINUE ITERATION, IF TEST EQUALS 0 GO TO NEXT
      C
      180 IF (TEST.EQ.1) GO TO 250
      IF (KT.GT.NUMT.OR.SUM.GT.TMAX) IFINAL=1
      NTH=C
      IF (MOD(KT,KTH).NE.0.OR.KT.EQ.0.AND.IFINAL.NE.1) GO TO 210
      WRITE (6,59C) KT,DELT,SUM,MINS,HRS,DAYS,KOUNT
      IF (CONTR.EQ.CURK(3)) CALL PRNTA
      IF (NU1.EQ.CHK(4)) CALL PRNT1
      IF (CHK.EQ.CHK(5)) CALL CHFCK
      IF (IFINAL.NE.1) GO TO 210
      IF (PNCH.NE.CHK(1)) STOP
      DO 200 I=1,DIML
      190 WRITE (7,520) (PHI(I,J),J=1,DIMW)
      WRITE (7,61C) SUM
      STOP
      210 CONTINUE
      KT=KT+1
      KOUNT=0
      DO 220 I=1,DIML
      DO 220 J=1,DIMW
      220 KEEPI,I,J)=PHI(I,J)
      IF (WATER.NE.CHK(2)) GO TO 240
      DO 230 I=1,DIML
      DO 230 J=1,DIMW
      T(I,J)=PERM(I,J)*(PHI(I,J)-BOTTOM(I,J))
      IF ((T(I,J).GE.0.) GO TO 230
      WRITE (6,66C)
      GO TO 190
      230 CONTINUE
      240 DELT=DELT+DELT
      SUM=SUM+DELT
      HRS=SUM/3600.
      MINS=HRS*60.
      DAYS=HRS/24.
      GO TO 270
      270 IF (KOUNT.LT.100) GO TO 260
      WRITE (6,670)
      GO TO 190
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A 250

260 KOUNT=KOUNT+1
IF (MOD(KOUNT,LENGTH)) 270,270,280
270 NTH=C
280 NTH=NTH+1
PARAV=RHOP(NTH)
TEST=0.
290 DO 300 J=1,DIMW
300 T=RP(J)=PHI(I,J)
DO 400 I=2,DIML
DO 360 J=2,JNOL
C
C
C DETERMINE WHETHER NODE IS OUTSIDE AQUIFER BOUNDARY
IF (T(I,J)) 310,36C,310
C
C
C 310 RH0=S(I,J)/DELT
C
C
C CALCULATE AVERAGE VALUES OF T BETWEEN ADJACENT NODES
C NODE CONFIGURATION: T1=LEFT, T2=RIGHT, T3=UPPER, T4=LOWER
T1=((2.*T(I,J-1)*T(I,J))/(T(I,J)*DELX(J-1)+T(I,J-1)*DELX(J)))
T2=((2.*T(I,J+1)*T(I,J))/(T(I,J)*DELX(J+1)+T(I,J+1)*DELX(J)))
T3=((2.*T(I-1,J)*T(I,J))/(T(I,J)*DELY(I-1)+T(I-1,J)*DELY(I)))
T4=((2.*T(I+1,J)*T(I,J))/(T(I,J)*DELY(I+1)+T(I+1,J)*DELY(I)))
IMK=PARAM*(T1+T2+T3+T4)
K=0,C
C
C
C CHECK WHETHER NODE IS ALONG A STREAM OR ON A LAKE
IF (RATE(I,J)) 330,33C,32C
320 K=RATE(I,J)
C
C
C CALCULATE VALUES FOR PARAMETERS A,B,C, AND BE
330 B=-T1-T2-RHO-K/(12.*M)-IMK
A=T1
C=T2
W=B-A*B*E(J-1)
BE(J)=C/W
C

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C
C   CHECK NUDE FOR POSSIBLE WELL LOCATION
C
C   RR=QRE
C   RW=0,0
C   IF (RATE(I,J)) .GT. 340,350,350
C   RW=-RATE(I,J)/(DEFLX(I,J)*DELY(I))
C   340  D=-T3*PHI(I-1,J)+(T4+T3-IMK)*PHI(I,J)-T4*PHI(I+1,J)-RHO*KEEP(I,J)-
C   1(K/W)*RIVERKEEP(I,J)/2.+RW-RR
C   C(J)=(D-L*G(J-1))/A
C   360 CONTINUE
C
C   CALCULATE HEAD VALUES FOR ROWS OF MATRIX AND PLACE THEM IN
C   TEMPORARY LOCATION TEMP
C   NU3=D1*W-2
C   DO 390 KND4=1,NO3
C   NO4=D1*W-KNO4
C   PHI(I-1,NO4)=TEMP(NO4)
C   IF (T(I,NO4)) 38C,37C,38C
C   370 TEMP(NO4)=PHI(I,NO4)
C   GO TO 390
C   380 TEMP(NO4)=G(NO4)-DE(NO4)*TEMP(NO4+1)
C   39C CONTINUE
C   40C CONTINUE
C
C   *****
C   FOLLOW SIMILAR PROCEDURE FOR COLUMNS OF MATRIX AS THAT CONSIDERED
C   FOR ROWS
C   DO 410 I=1,DIML
C   410 TEMP(I)=PHI(I,1)
C   INGL=DIML-1
C   DO 510 J=2,DIMN
C   510 I=2,IND1
C   IF (T(I,J)) 420,470,420
C
C   420 RHO=S(I,J)/DELT
C
C   CALCULATE AVERAGE VALUES OF T BETWEEN ADJACENT NODES
C   T1=(2.*T(I,J-1)*T(I,J))/(T(I,J-1)*DELX(J-1)+T(I,J)*DELX(J))/DELT
C   1(J)
C   T2=((2.*T(I,J+1)*T(I,J))/(T(I,J+1)*DELX(J+1)+T(I,J)*DELX(J))/DELT
C   1(J)
C   T3=((2.*T(I-1,J)*T(I,J))/(T(I,J)*DELY(I-1)+T(I,J)*DELY(I))/DELT A 300

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1(I)
T4=((2.*T(I+1,J)*T(I,J))/(T(I,J)*DELY(I+1,J)+T(I+1,J)*DELY(I)))/DELY
(V1(I))
IMK=PARAM*(T1*T2*T3+T4)
K=0. C
C
C
C CHECK WHETHER NODE IS ALONG A STREAM OR ON A LAKE
C IF (RATE(I,J) .GT. 440,440,430
430,K=RATE(I,J)
C
C
C CALCULATE VALUES FOR PARAMETERS A,B,C,AND BE
440 A=T3
C=T4
B=-T4-T3-RHO-K/(2.*M)-IMK
W=B-A*B*(I-1)
BE(I)=C/N
C
C
C CHECK NODE FOR POSSIBLE WELL LOCATION
450 QRE
RW=0.0
IF (RATE(I,J) .GT. 450,460,460
450 RN=-RATE(I,J)/(DELX(J)*DELY(I))
460 D=-T1**PHI(I,J-1)+(T1+T2-1NK)*PHI(I,J)-T2*PHI(I,J+1)-RHO*KEEP(I,J)-
1(K/M)*(RIVFR-KEFP(I,J)/2.0)+RW-RR
G(I)=(D-A*G(I-1))/N
470 CONTINUE
C
C
C CALCULATE HEAD VALUES FOR COLUMNS OF MATRIX AND PLACE IN TEMPORARY
C LOCATION TEMP
N03=D*ML-2
DO 500 KNO4=1,N03
N04=DIML-KNO4
PHI(N04,J-1)=TEM0(N04)
IF ((T(N04,J)) .GT. 490,480,490
480 TEMP(N04)=PHI(N04,J)
GO TO 500
490 TEMP(N04)=G(N04)-BE(N04)*TEMP(N04+1)
IF (DABS(TEMP(N04)-PHI(N04,J)) .GT. ERR) TEST=1.
500 CONTINUE
510 CONTINUE
GO TO 100
C

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***** C 520 FORMAT (F10.2,31I0,2F10.2,F10.2,I10,F10.2,I10,4F10.2/3F10.2) A 351
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      A 387
      A 388
      A 389
      A 390-
C 530 FORMAT (1H1,6I4,C) C
C 540 FORMAT (1H1,6I4,11H RATE MATRIX)
C 550 FORMAT (1H1,6I4,16H INPUT PARAMETERS//40H LENGTH OF INITIAL TIME STEP)
C 550 'FOR MAX 1H1,6I4,16H INPUT PARAMETERS//40H LENGTH OF INITIAL TIME STEP' A 358
C 550 'STEP IN SECONDS=,E10.3//46H MAXIMUM PERMITTED PERIOD OF PUMPING IN H A 359
C 550 'DAYS=,E10.3//4CH MAXIMUM PERMITTED NUMBER OF TIME STEPS=,I4//6CH F A 360
C 550 'BLUX DUE TO VERTICAL LEAKAGE FROM A CONFINING LAYER IN CFS=,E10.3// A 361
C 550 '437H NUMBER OF NODES IN COLUMN OF MATRIX=,I4//34H NUMBER OF NODES I A 362
C 550 '54 ROW OF MATRIX=,I4//32H NUMBER OF ITERATION PARAMETERS=,I4//28H E A 363
C 550 '6940P CRITERIA FOR CLOSURE=,E10.3//53H MULTIPLIER FOR HYDRAULIC CON A 364
C 550 'DUCTIVITY OF STREAM BED=,E10.3//36H MULTIPLIER FOR STORAGE COEFFICIENT A 365
C 550 'AFFECT=,E10.3//34H MULTIPLIER FOR BOTTOM ELEVATION=,E10.3//39H MULTI A 366
C 550 'PLIER FOR HYDRAULIC CONDUCTIVITY=,E10.3//31H MULTIPLIER FOR TRANSM A 367
C 550 'ISSIBILITY=,E10.3//15H HEAD IN RIVER=,E10.3//25H THICKNESS OF STREAM A 368
C 550 'BED=,E10.3//40H NUMBER OF TIME STEPS BETWEEN PRINTOUTS=,I4/ A 369
C 550 FORMAT (1H1,6I4,23H TRANSMISSIBILITY MATRIX) A 370
C 550 FORMAT (1H0,15,(1H ,14E9,1)) A 371
C 550 FORMAT (1H1,5I4,17H TIME STEP NUMBER=,I10/50X,29HSIZE OF TIME STEP A 372
C 550 '11, SEC/ENDS=,E10.2/40X,48HDURATION OF PUMPING AT THIS PRINTOUT IN S A 373
C 550 'SECONDS=,E10.3/86X,8HMINUTES=,E10.3/86X,6HHOURS=,E10.3/86X,5HDAYS=, A 374
C 550 '3E10.3/56X,17HITERATION NUMBER=,I10) A 375
C 550 FORMAT (1H1,5I4,20H STORAGE COEFFICIENT MATRIX) A 376
C 550 FORMAT (A5/A8/A7/A7/A5) A 377
C 550 FORMAT (F10.3) A 378
C 550 FORMAT (8F10.4) A 379
C 550 FORMAT (1H1,52X,29HYDRAULIC CONDUCTIVITY MATRIX) A 380
C 550 FORMAT (1H1,46X,40HIMPERMEABLE BASE OF AQUIFER) A 381
C 550 FORMAT (1H1,56X,20HITERATION PARAMETERS//(1H ,10E12.3)) A 382
C 550 FORMAT (1H0,13HWHEN GOES DRY) A 383
C 550 FORMAT (1H0,39HITCEEDED PERMITTED NUMBER OF ITERATIONS) A 384
C 550 FORMAT (F10.0) A 385
C 550 FORMAT (1H1,40X,40HGRID SPACING IN PROTOTYPE IN X DIRECTION//(1H0 A 386
C 550 '1,12F10.3)) A 387
C 550 'FOR MAX 1H1,40X,40HGRID SPACING IN PROTOTYPE IN Y DIRECTION//(1H0 A 388
C 550 '1,12F10.3)) A 389
C 550 'FOR MAX 1H1,40X,40HGRID SPACING IN PROTOTYPE IN Z DIRECTION//(1H0 A 390-

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C **** SUBROUTINE PRNTA
C **** THIS SUBROUTINE PRINTS OUT THE HEAD MATRIX AS ALPHABETIC CONTOURS
C COMMON PHI,SUM,DELX,DELY,DIML,DIMW,RATE,S,STRT,SPACNG
C INTEGER DIAL,DIMW
C REAL K
C DIMENSION RATE(50,50), SYM(39), PRNT(60), PHI(50,50), STRT(50,50),
C 1'BLANK(50), S(50,50)
C DOUBLE PRECISION PHI
C
C WRITE (6,50)
C DATA SYM/1H1,1H2,1H3,1H4,1H5,1H6,1H7,1H8,1H9,1HA,1HB,1HC,1HD,1HE,1
C 1HF,1HG,1HH,1HI,1HJ,1HK,1HL,1HM,1IN,1HO,1HP,1HQ,1HS,1HT,1HU,1HV
C 2,1HW,1HX,1HY,1HZ,1HO,1H ,1H#,1HG/,BLANK/60*1H /
C 51H0=(65-DIMW)/2
C DO 40 I=1,DIML
C DO 30 J=1,DIMW
C K=STRT(I,J),JL)-PHI(I,J),JB)
C K=N/SPACNG
C IF (K.LT.C) GO TO 1C
C K=A MOD(K,36.)
C
C 10 IF (K.LT.1.) PRNT(JB)=SYM(36)
C 10 IF (K.LT.C) PRNT(JR)=SYM(39)
C 10 IF (PRNT(I,JR).EQ.STRT(I,JB)) PRNT(JB)=SYM(37)
C N=K
C
C IF (N.LT.1) GO TO 20
C PRNT(JB)=SYM(N)
C 20 IF (RATE(IH,JB).GT.0.) PRNT(JB)=SYM(27)
C 20 IF (RATE(IH,J3).LT.0.) PRNT(JD)=SYM(32)
C 30 CONTINUE
C 40 WRITE (6,60) (BLANK(I),I=1,IND), (PRNT(JB),JB=1,DIMW)
C WRITE (6,70) SPACNG
C RETURN
C
C **** **** **** **** **** **** **** **** **** **** ****
C
C 50 FORMAT (1HC,50X,32HALPHABETIC CONTOURS FOR DRAWDOWN,////)
C 60 FORMAT (1H *65A2)
C 70 FORMAT (1CH0LEGEND**/18HC CONTOUR INTERVAL=,F10.3/32H LOCATION OF
C 1RECHARGE BOUNDARY=R/16HC WELL LOCATION=W/21HC ONE OF IMPRESSION=G)
C END

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C **** SUBROUTINE PRNTI ****
C THIS SUBROUTINE PRINTS OUT THE HEAD MATRIX IN NUMERICAL FORM
C
C COMMON PHI, SUM, DFLX, DELY, DIML, DIMW, RATE, S, STRT, SPACNG
C
C DIMENSION RATE(50,50), DDM(50,50), PHI(50,50), S(50,50), STRT(50,50)
C
C INTEGER DIML, DIMW
C
C DOUBLE PRECISION PHI
C
C WRITE (6,30)
C
C 30 FORMAT (1H1,58X,16HDRAWDOWN IN FEET//)
C
C 40 FORMAT (1H0,15,(1H ,1HE11.3))
C
C END
C
C ****
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***** SUBROUTINE CHECK *****

THIS SUBROUTINE COMPUTES THE ERROR IN THE SOLUTION ON A MASS
BALANCE BASIS

*****WARNING - USE THIS SUBROUTINE ONLY WHEN THERE IS NO INFILTRATION
INTO THE SYSTEM****

COMMON PHI,SUM,DELX,DELY,DINW,RATE,S,STRT,SPACNG
      DIML,DINW
      DIMENSION RATE(50,50), S(50,50), PHI(50,50),
      STRT(50,50)
      DIMENSION DELY(50), DELX(50)
      DOUBLE PRECISION PHI
      TOTAL=0.0
      PUMP=0.0
      DO 10 IC=2,DIML
      DO 10 JC=2,DINW
      TCTL=STUTL+S(IC,JC)*DELX(JC)*DELY(IC)*(STRT(IC,JC)-PHI(IC,JC))
10     IF (RAYE(IC,JC).LT.0.0) PUMP=PUMP-RATE(IC,JC)*SUM
      DIFF=PUMP-TOTAL
      PERCENT=(DIFF/PUMP)*100.
      WRITE (6,20) TOTAL,PUMP,DIFF,PERCENT
      RETURN

***** FORMATTED OUTPUT *****
20 FORMAT (63H0QUANTITY PUMPED ACCORDING TO CONE OF DEPRESSION IN CUB
11IC FEET,,E20.10//59H QUANTITY PUMPED ACCORDING TO WELL DISCHARGE I
2N CUBIC FEET,,E20.10//51H ESTIMATE FROM PUMPING LESS ESTIMATE FROM
3 DRAWDOWN,,E20.10//42H DIFFERENCE AS A PERCENT OF VOLUME PUMPED,,E
420.1C)
      END

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